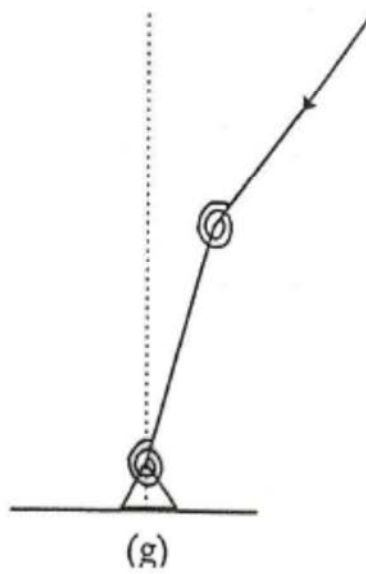
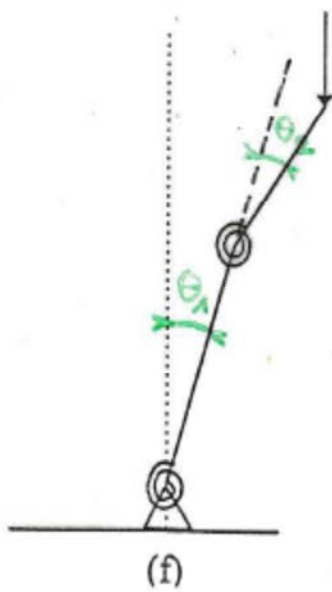
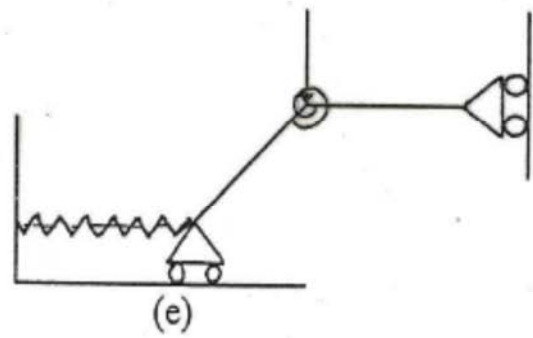
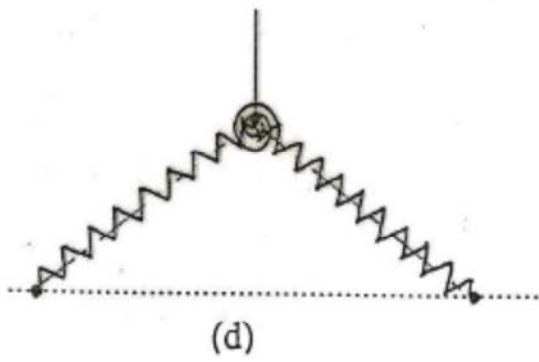
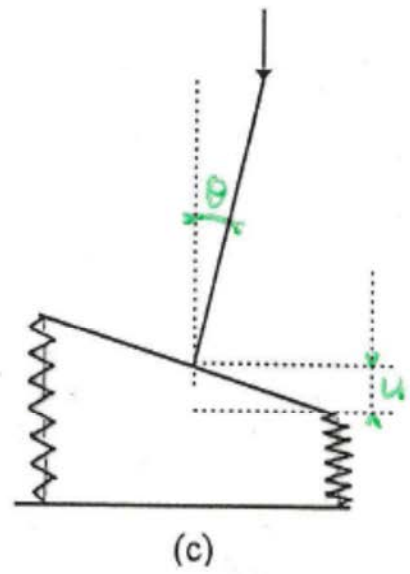
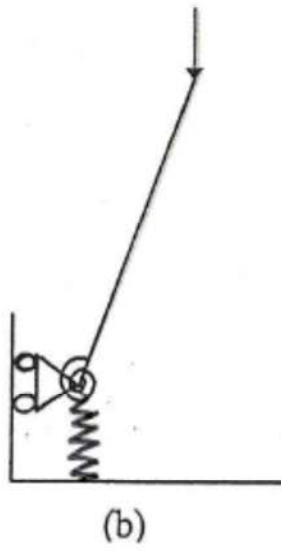
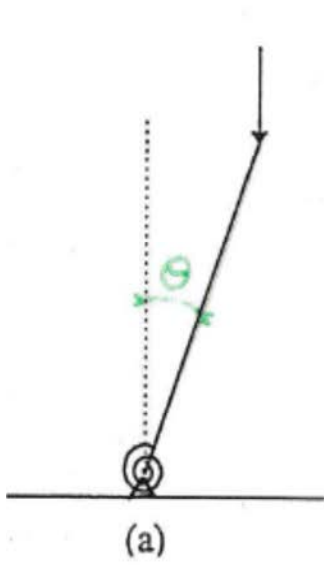
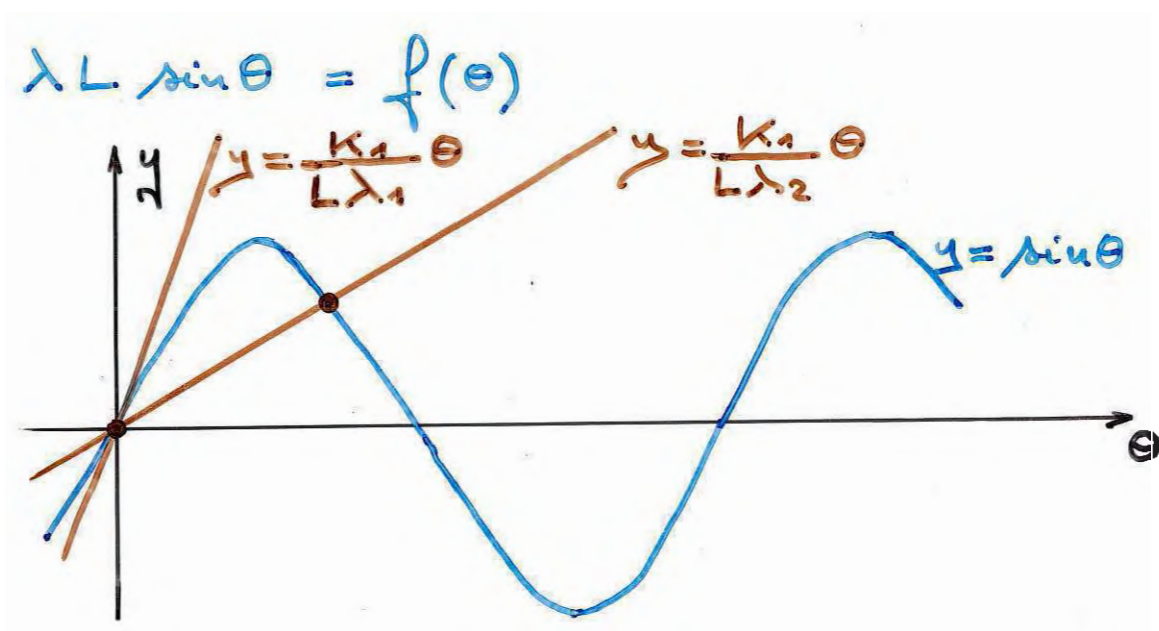
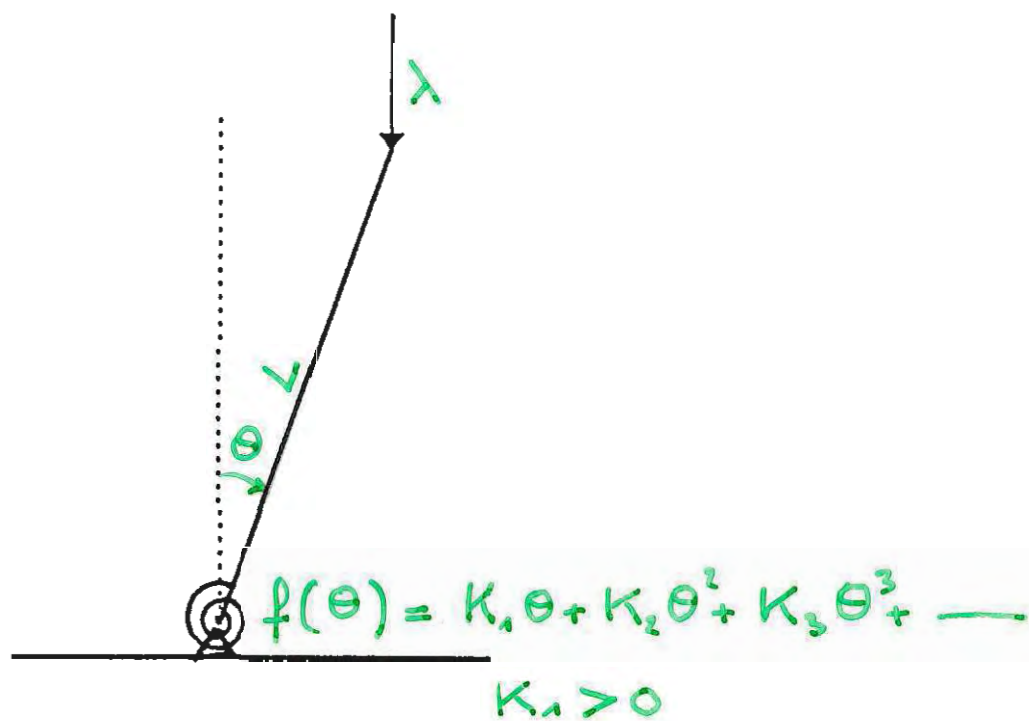
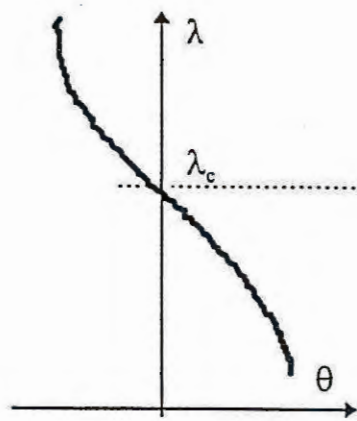


SIMPLE MODELS AND STRUCTURES

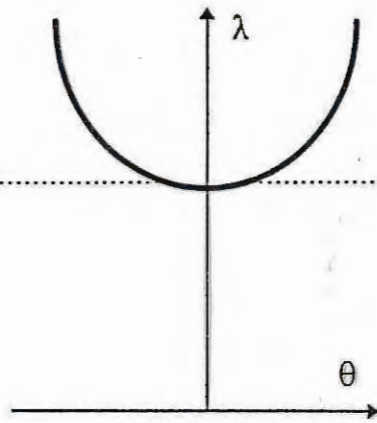




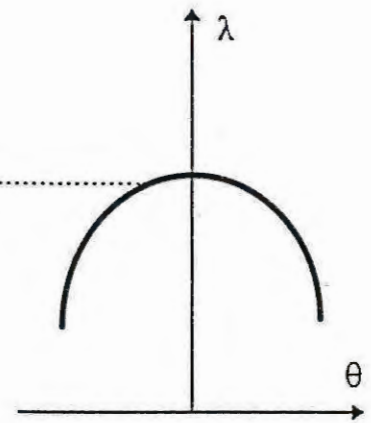
$$\begin{cases} \theta = 0, \forall \lambda \in \mathbb{R}^+ \\ \lambda = \frac{f(\theta)}{L \sin \theta} \end{cases}$$



(a) $K_2 < 0$



(b) $K_2 = 0, \lambda_1 = 0, \lambda_2 > 0$

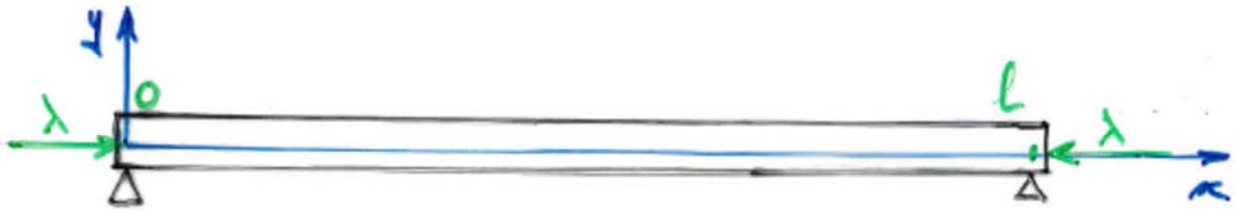


(c) $K_2 = 0, \lambda_1 = 0, \lambda_2 < 0$

$$\lambda = \lambda_c + \lambda_1 \theta + \lambda_2 \theta^2 + \dots$$

$$\left\{ \begin{array}{l} \lambda_c = \frac{K_2}{L} \\ \lambda_1 = \lambda_c \frac{K_2}{K_1} \\ \lambda_2 = \lambda_c \left[\frac{K_3}{K_1} + \frac{1}{6} \right] \\ \vdots \end{array} \right.$$

THE COMPRESSED BEAM



Kinematics:

$$\begin{cases} \varepsilon(x) = u' + \frac{(v')^2}{2} \\ \kappa(x) = -v'' \end{cases}, \quad (\cdot)' \equiv \frac{d}{dx}$$

$$\hookrightarrow \gamma(x, y) = u'(x) + \frac{(v'(x))^2}{2} - y v''(x)$$

Equilibrium:

$$\begin{cases} N' = 0 \\ M'' + N\kappa = 0 \end{cases}$$

Constitutive law:

$$\begin{cases} N = ES \left(u' + \frac{(v')^2}{2} \right) \\ M = EI v'' \end{cases}$$



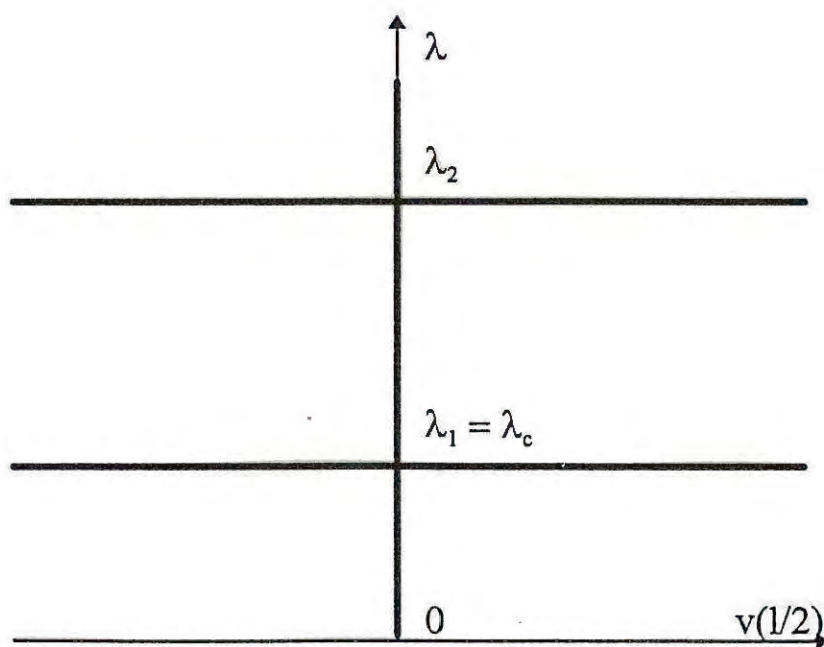
$$\begin{aligned} N &= C_{sta} = -\lambda \\ (EI v'')'' + \lambda v'' &= 0 \\ &+ \text{B.C.} \end{aligned}$$

- * Uniform cross-section
- * S. S. Boundary Conditions

$$\begin{cases} EI v'' + \lambda v = 0 \\ v(0) = v(l) = 0 \end{cases}$$

$$\Rightarrow \lambda_n = \frac{n^2 \pi^2 EI}{l^2}$$

\Rightarrow Bifurcation diagram:

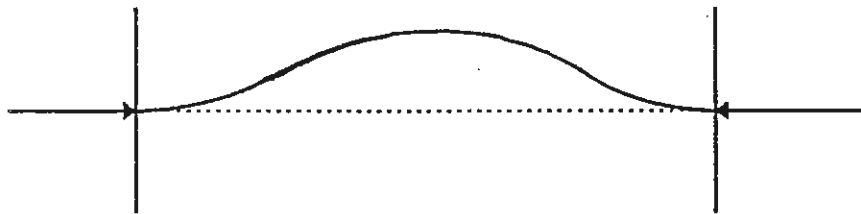


- Extrémités simplement appuyées:



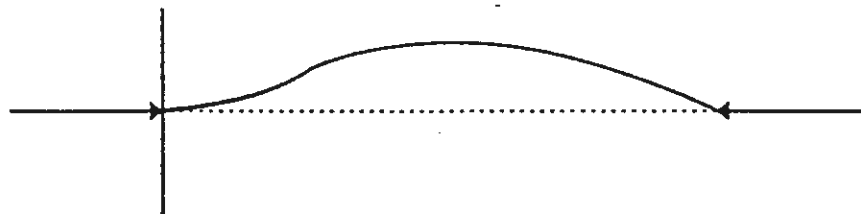
$$\lambda_c = \frac{\pi^2 EI}{\ell^2}$$

- Extrémités encastées:



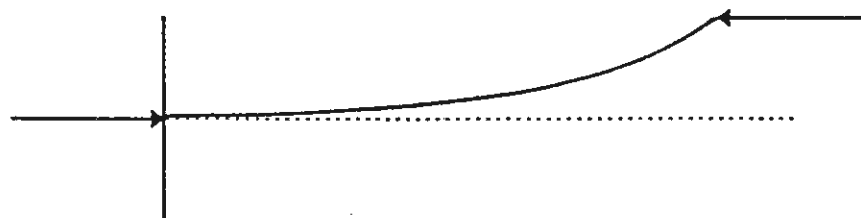
$$\lambda_c = \frac{4\pi^2 EI}{\ell^2}$$

- Appui simple à une extrémité, encastrement à l'autre:

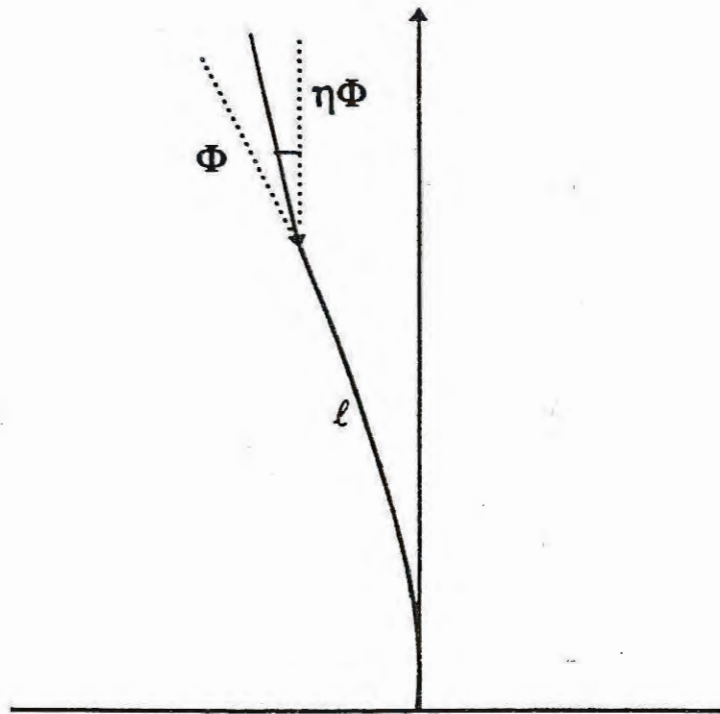


$$\lambda_c = 2.04 \frac{\pi^2 EI}{\ell^2}$$

- Une extrémité encastree, l'autre libre:



$$\lambda_c = \frac{\pi^2 EI}{4\ell^2}$$



$$EI \nu_{xxxx} + \lambda \nu_{xx} + m \nu_{tt} = 0$$

$$\nu(x,t) = e^{i\omega t} f(x) \Rightarrow \begin{cases} EI f^{(4)} + \lambda f'' - m\omega^2 f = 0 \\ f(0) = f'(0) = f''(L) = 0 \\ f'''(L) - (\eta - 1) \frac{\lambda}{EI} f'(L) = 0 \end{cases}$$

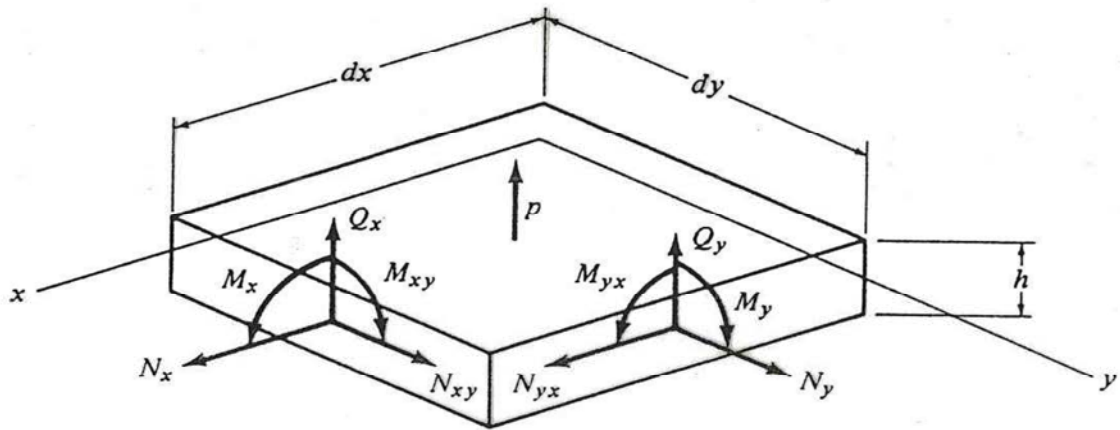
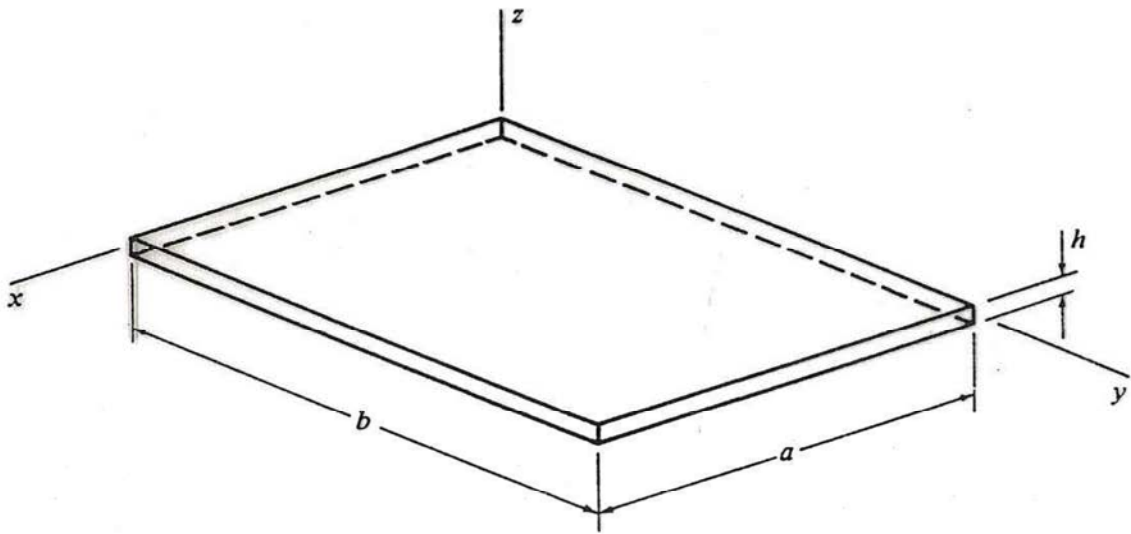
1° $\omega = 0$:

$$\lambda_c = \frac{\pi^2 EI}{L^2} \left[\frac{1}{\pi^2} \left[\arccos \frac{\eta}{\eta - 1} \right]^2 \right]$$

$\Rightarrow \eta \leq 0.5!$

2° $\eta > 0.5$ $\Rightarrow \omega^2 = 1.24 \frac{\pi^2 EI}{m L^4}$ \hat{a} $\lambda = 2.005 \frac{EI}{L^2}$

THE VON KARMAN PLATE



Kirchhoff - Love kinematics:

$$\begin{cases} \bar{u} = u + \frac{h}{2} \beta_x \\ \bar{v} = v + \frac{h}{2} \beta_y \\ \bar{w} = w \end{cases} \quad \text{where} \quad \begin{cases} \beta_x = -w_{,x} \\ \beta_y = -w_{,y} \end{cases}$$

u, v, w : fcts of (x, y) only.

* Small strains, finite displacements

$$\left\{ \begin{array}{l} \epsilon_x = u_{,x} + \frac{1}{2}(w_{,x})^2 \\ \epsilon_y = v_{,y} + \frac{1}{2}(w_{,y})^2 \\ \gamma_{xy} = u_{,y} + v_{,x} + w_{,x}w_{,y} \end{array} \right. \quad \left\{ \begin{array}{l} \kappa_x = -w_{,xx} \\ \kappa_y = -w_{,yy} \\ \kappa_{xy} = -w_{,xy} \end{array} \right.$$

* Constitutive law:

$$\left\{ \begin{array}{l} N_x = C(\epsilon_x + \nu \epsilon_y) \\ N_y = C(\epsilon_y + \nu \epsilon_x) \\ N_{xy} = C \frac{1-\nu}{2} \gamma_{xy} \end{array} \right. \quad \left\{ \begin{array}{l} M_x = D(\kappa_x + \nu \kappa_y) \\ M_y = D(\kappa_y + \nu \kappa_x) \\ M_{xy} = D(1-\nu) \kappa_{xy} \end{array} \right.$$

with:

$$C \equiv \frac{Eh}{1-\nu^2} ; \quad D \equiv \frac{Eh^3}{12(1-\nu^2)}$$

* Equilibrium: Von Kármán equations

$$N_{x,x} + N_{xy,y} = 0$$

$$N_{xy,x} + N_{y,y} = 0$$

$$D \nabla^4 w - (N_x w_{,xx} + 2 N_{xy} w_{,xy} + N_y w_{,yy}) = p$$

REMARK:

- * Let's introduce a "stress function" $f(x, y)$ by:

$$\begin{cases} N_x = f_{,yy} \\ N_y = f_{,xx} \\ N_{xy} = -f_{,xy} \end{cases}$$

- * and a bracket (Monge - Ampère form):

$$[f, g] \stackrel{\text{def}}{=} f_{,xx} g_{,yy} + f_{,yy} g_{,xx} - 2 f_{,xy} g_{,xy}$$

- * Then the set of von Kármán equations is changed into:

$$\nabla^4 f = -Eh[w, w]$$

$$D \nabla^4 w = [f, w] + p$$

+

B. C.

Assume for a given load p , there exists a solution (u_0, v_0, w_0) corresponding to N_{x0}, N_{y0}, \dots

* "Adjacent" equilibrium :

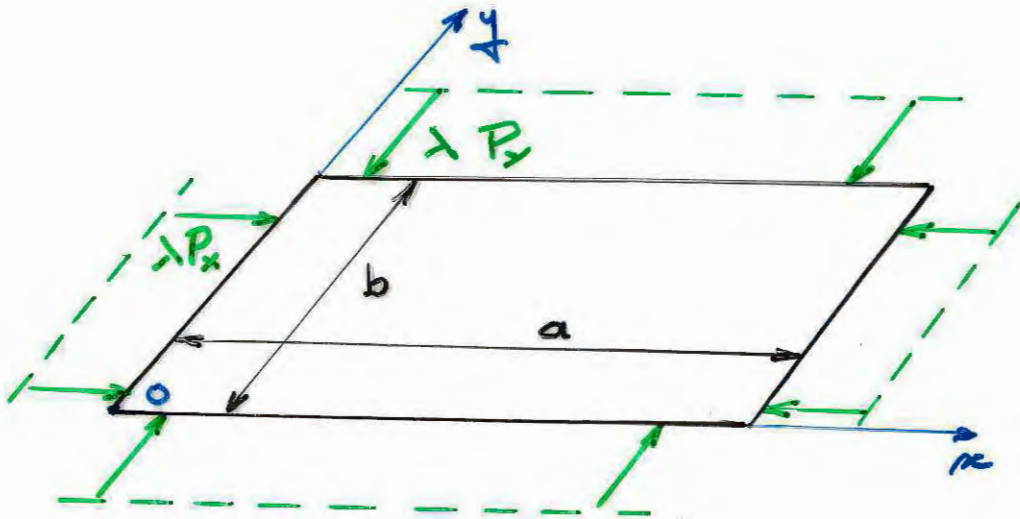
$$\exists? (\tilde{u}, \tilde{v}, \tilde{w}) \neq 0 \ \&$$

$u = u_0 + \tilde{u}$, $v = v_0 + \tilde{v}$, $w = w_0 + \tilde{w}$
is also a solution for the same p

Then:

$$\begin{aligned} \tilde{N}_{x,x} + \tilde{N}_{xy,y} &= 0 \\ \tilde{N}_{xy,x} + \tilde{N}_{y,y} &= 0 \\ D \nabla^4 \tilde{w} - (N_{x0} \tilde{w}_{,xx} + 2 N_{xy0} \tilde{w}_{,xy} + N_{y0} \tilde{w}_{,yy}) &= 0 \end{aligned}$$

- Homogeneous
- Uncoupled
- Linear.



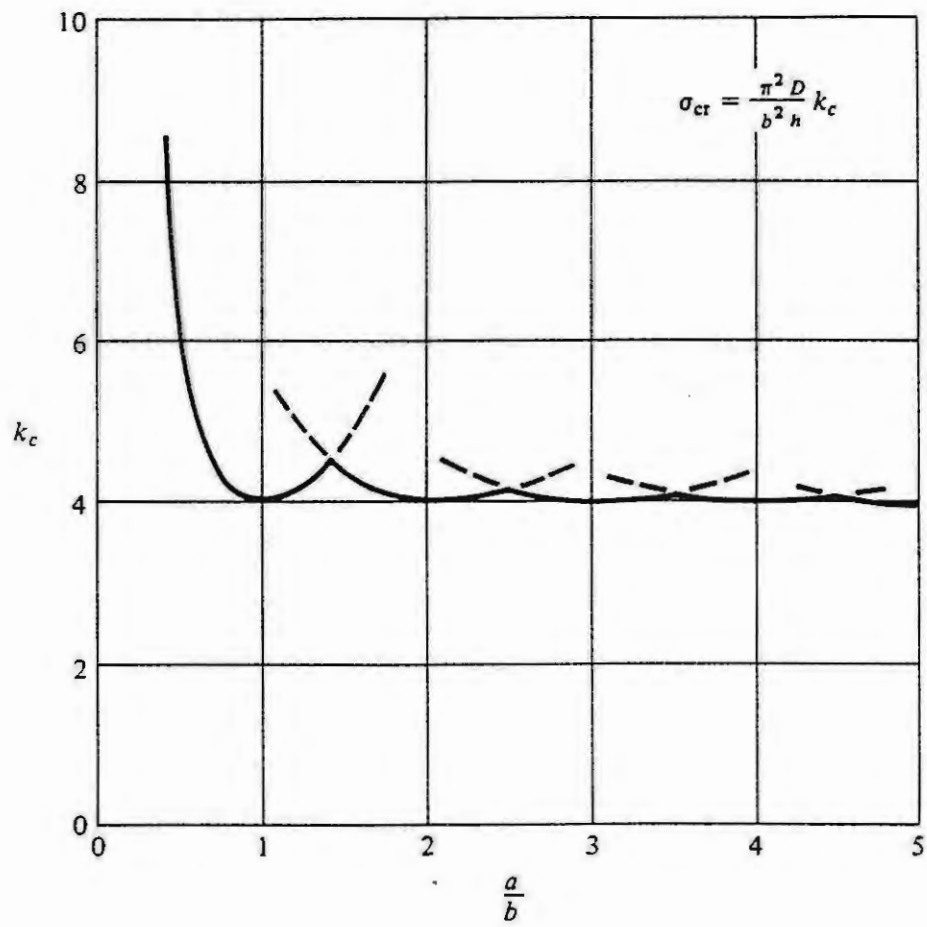
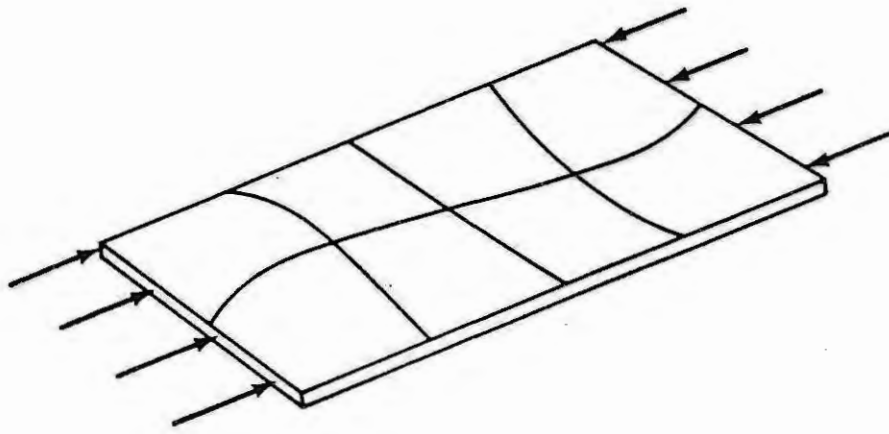
$$\begin{cases} N_{x0} = -\lambda P_x; N_{y0} = -\lambda P_y; N_{xy0} = 0; \\ D \nabla^4 w + \lambda P_x w_{,xx} + \lambda P_y w_{,yy} = 0 \\ w = w_{,xx} = 0 \text{ en } x=0 \text{ et } x=a \\ w = w_{,yy} = 0 \text{ en } y=0 \text{ et } y=b \end{cases}$$

$$* w(x, y) = \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$

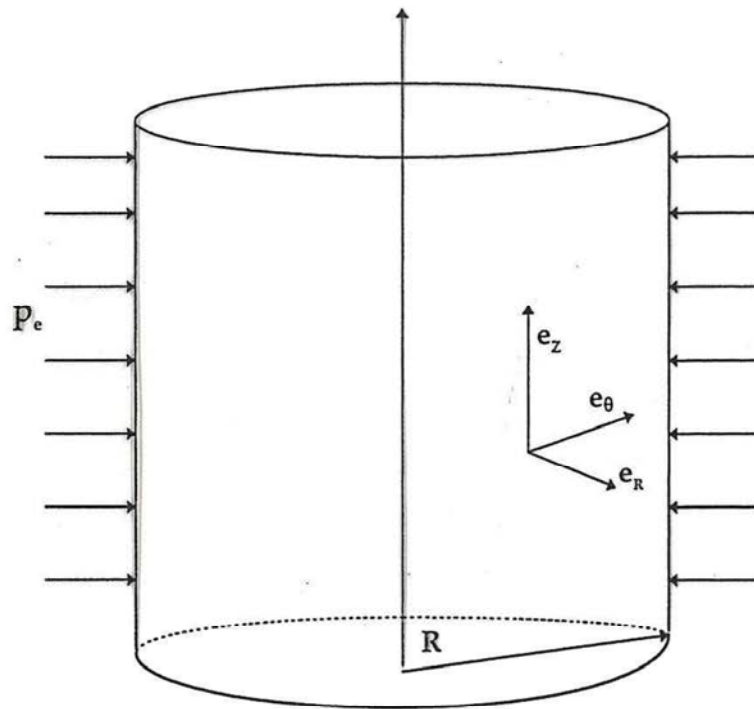
$$\hookrightarrow \lambda_c = k \pi^2 D, \quad k = \frac{\left[\left(\frac{m}{a}\right)^2 + \left(\frac{n}{b}\right)^2 \right]^2}{\left(\frac{m}{a}\right)^2 P_x + \left(\frac{n}{b}\right)^2 P_y}$$

$$* P_y = 0 \rightarrow n = 1$$

$$\lambda_c = k \frac{\pi^2 D}{b^2}, \quad k = \left(\frac{mb}{a} + \frac{a}{mb} \right)^2 \frac{1}{P_x}$$



CYLINDRICAL SHELLS



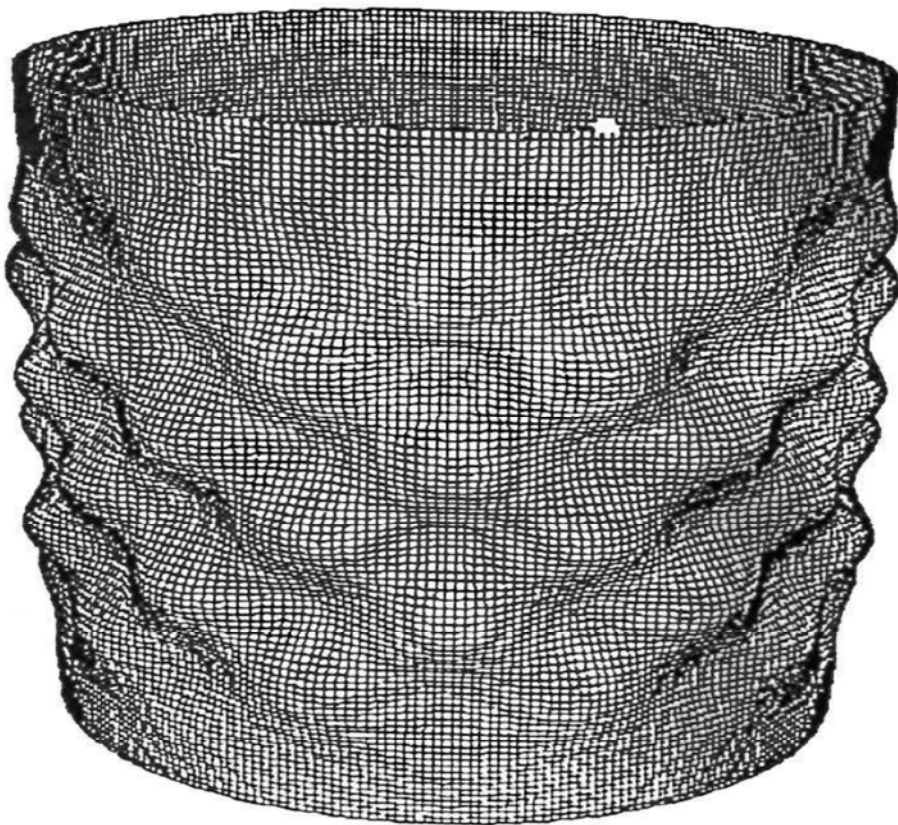
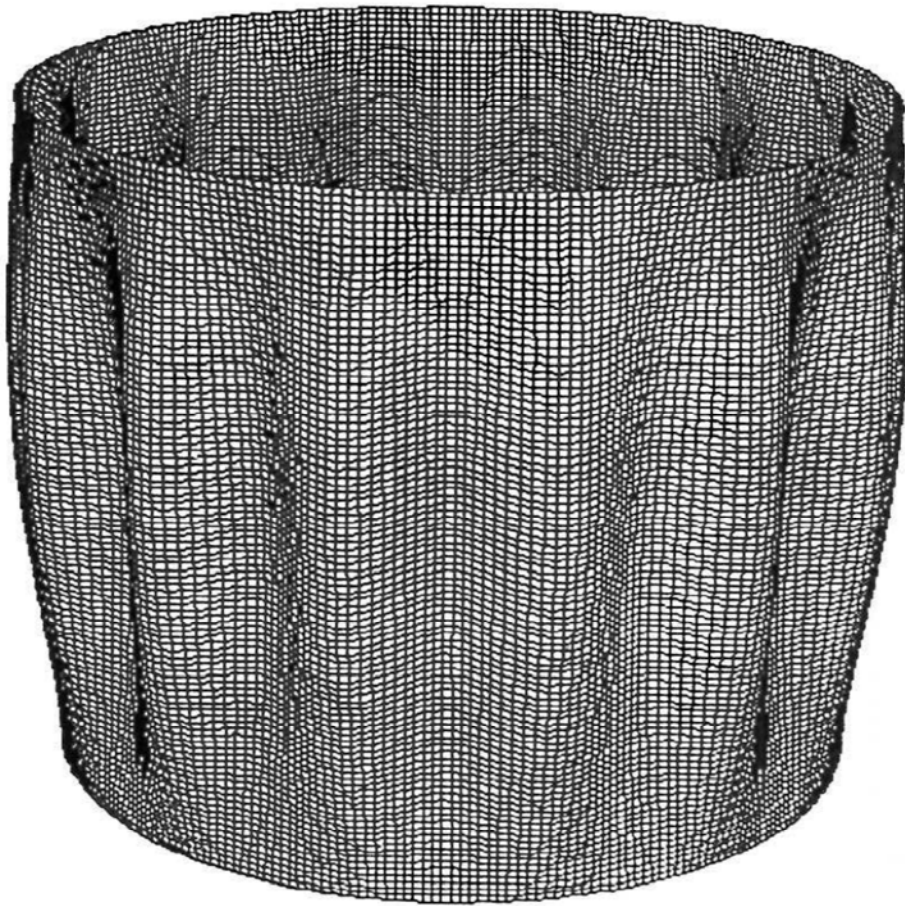
$$\begin{cases} D \nabla^4 w + R f_{,33} - [f_{,00} w_{,33} - 2 f_{,30} w_{,30} + f_{,33} w_{,00}] = P \\ \nabla^4 f - \frac{Eh}{R^4} [w_{,30}^2 - w_{,33} w_{,00} + R w_{,33}] = 0 \end{cases}$$

Perturbation autour de $N_\theta = -P_e R$:

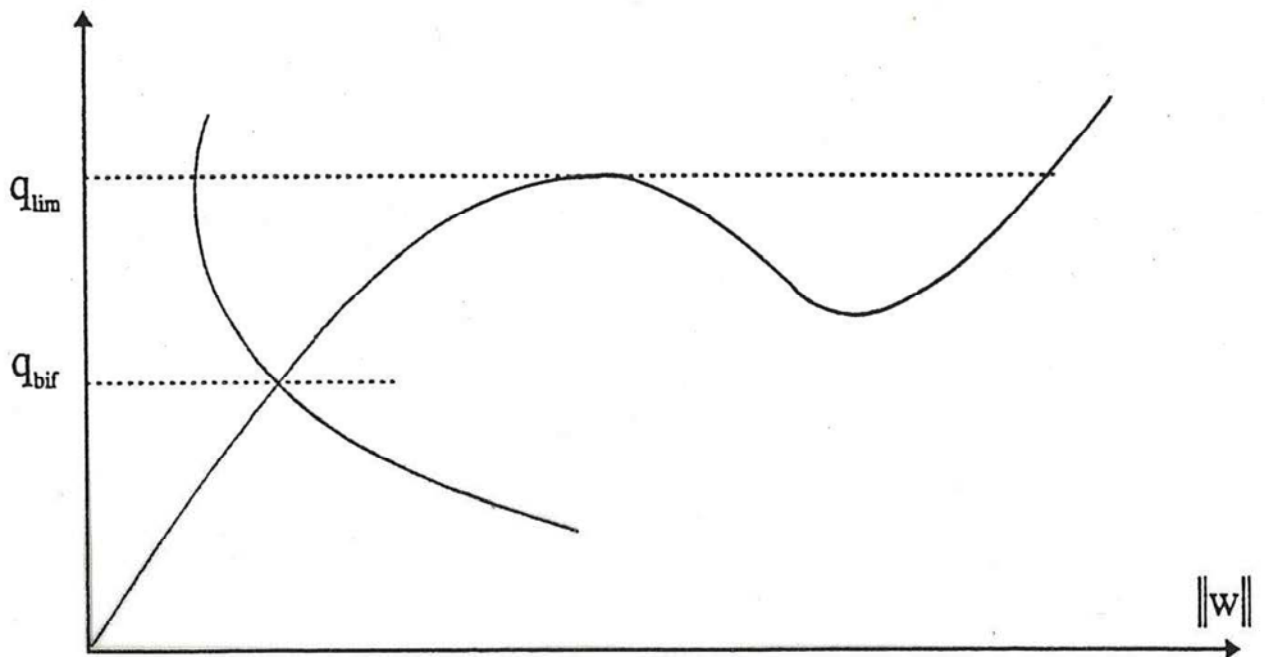
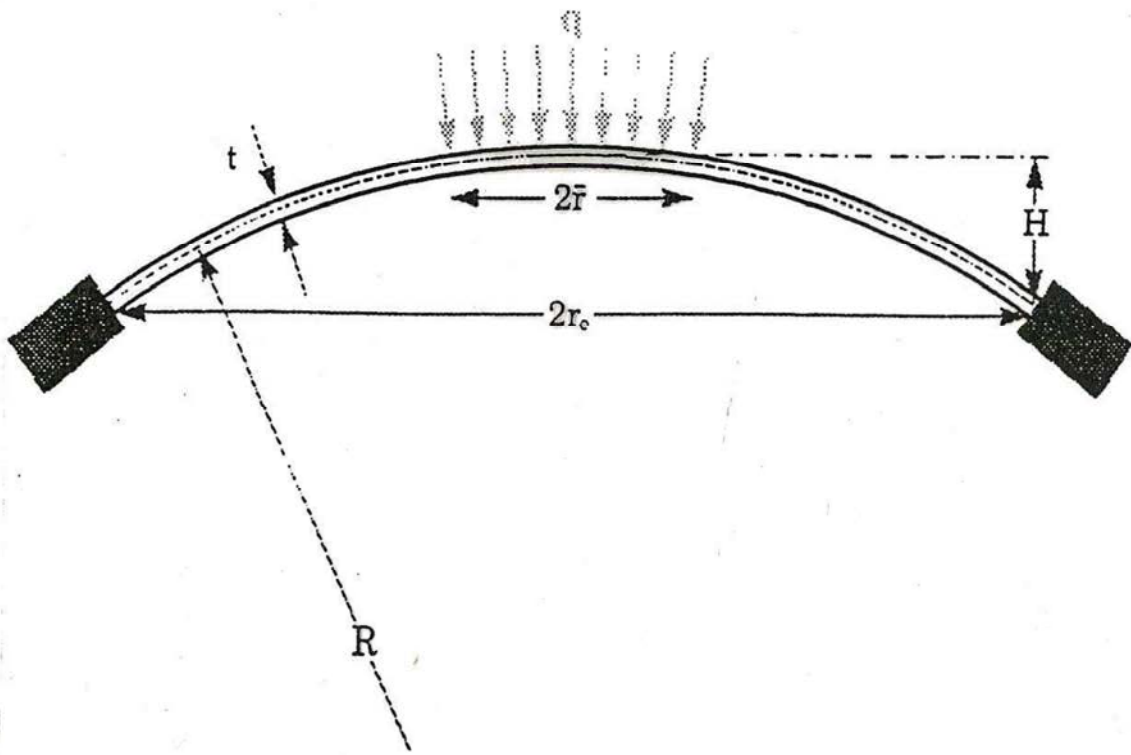
$$\begin{cases} D \nabla^4 w + \frac{1-\nu^2}{R^2} C w_{,max}^{(4)} + \frac{1}{R} P_e \nabla^4 w_{,00} = 0 \\ + \text{C.L.} \end{cases}$$

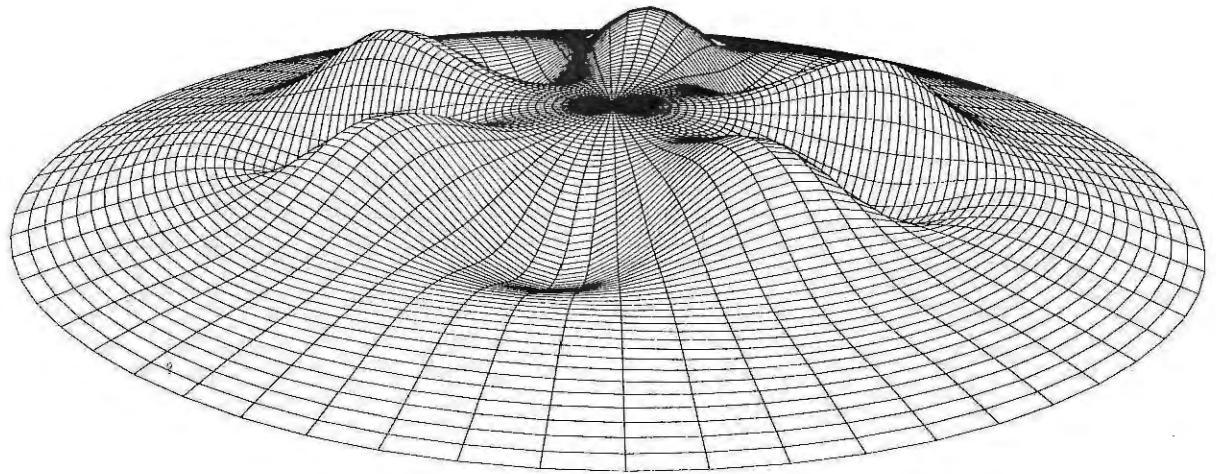
$$\rightarrow w = A \sin \bar{m} x \cdot \sin n \theta, \quad \bar{m} = \frac{m \pi R}{L}$$

$$\rightarrow \boxed{P_e R = \frac{(\bar{m}^2 + n^2)^2}{n^2} \frac{D}{R} + \frac{\bar{m}^4}{n^2 (\bar{m}^2 + n^2)^2} (1-\nu^2) C}$$

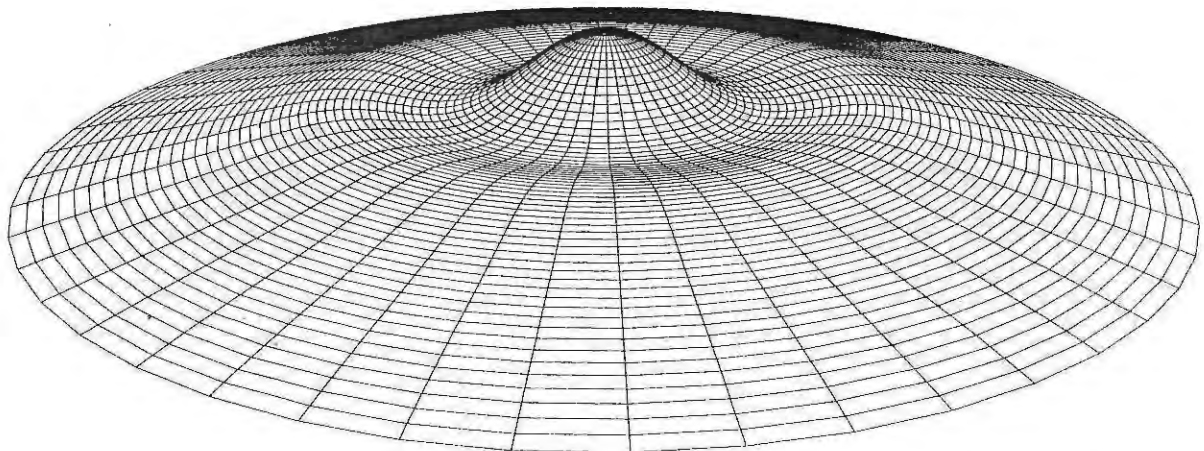


SPHERICAL CAP



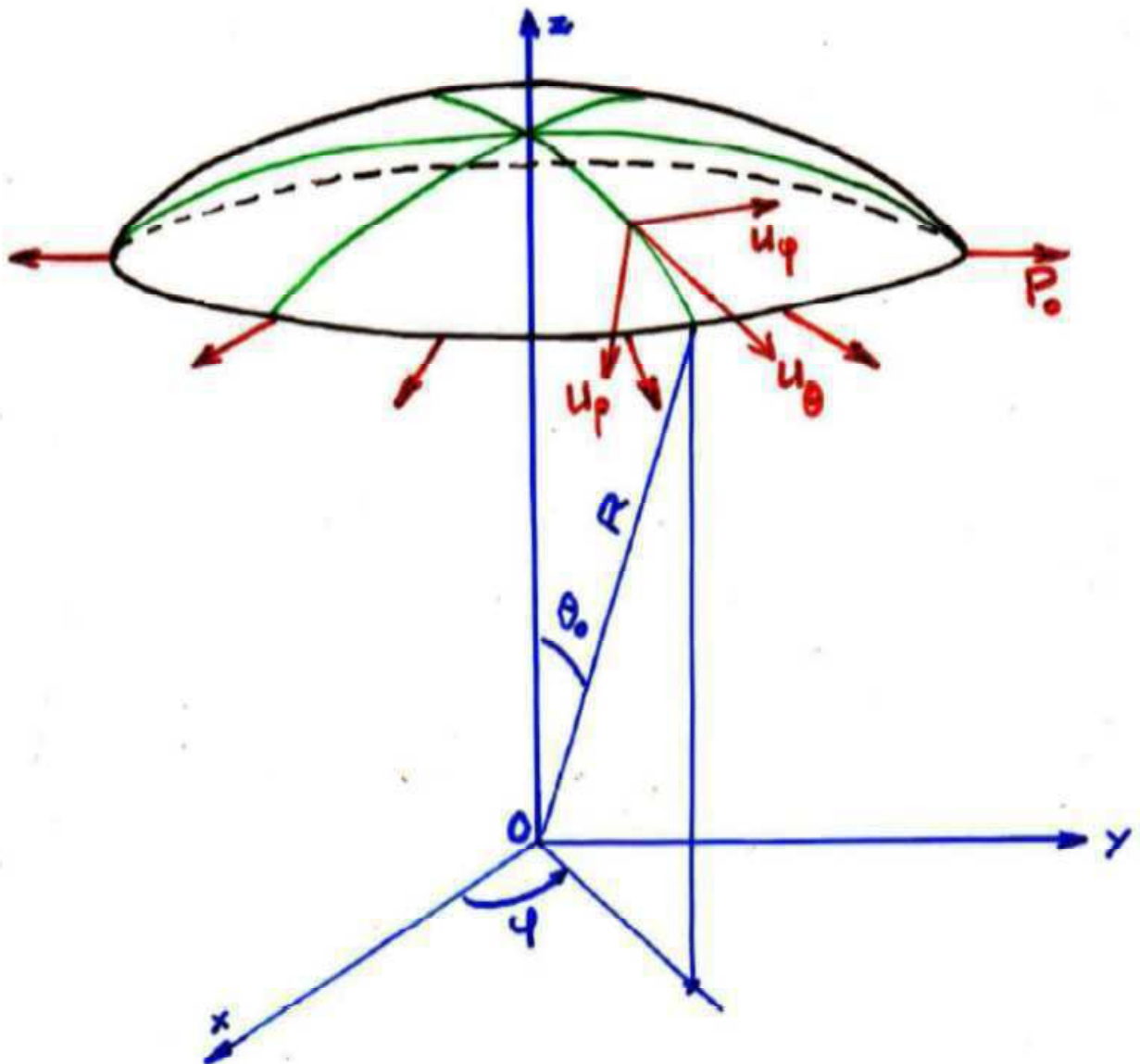


ALU - $\lambda_{\text{lamda}} = 9$ - $\lambda_{\text{lamda barre}} = 8.4$ - flambage en mode 5



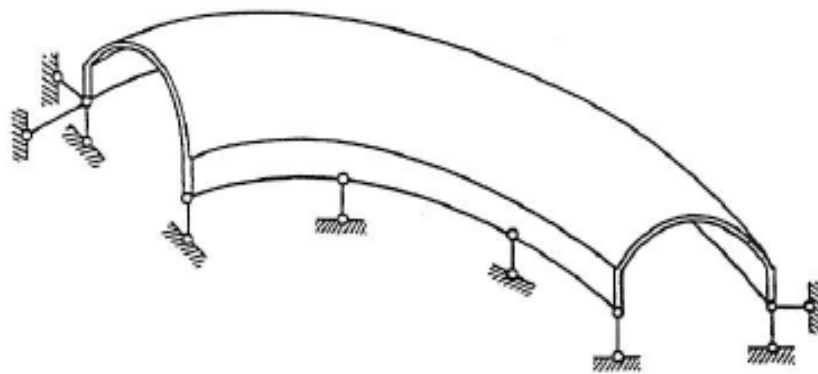
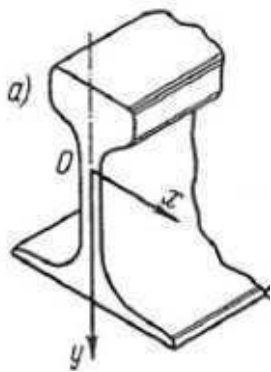
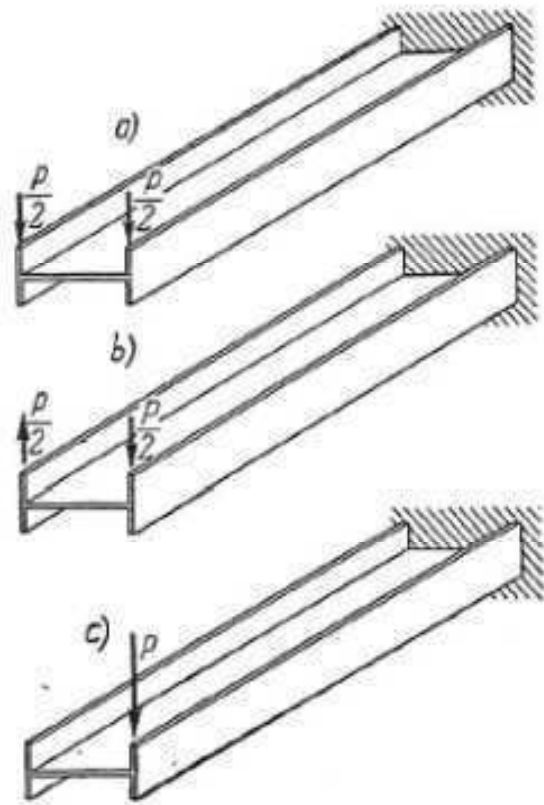
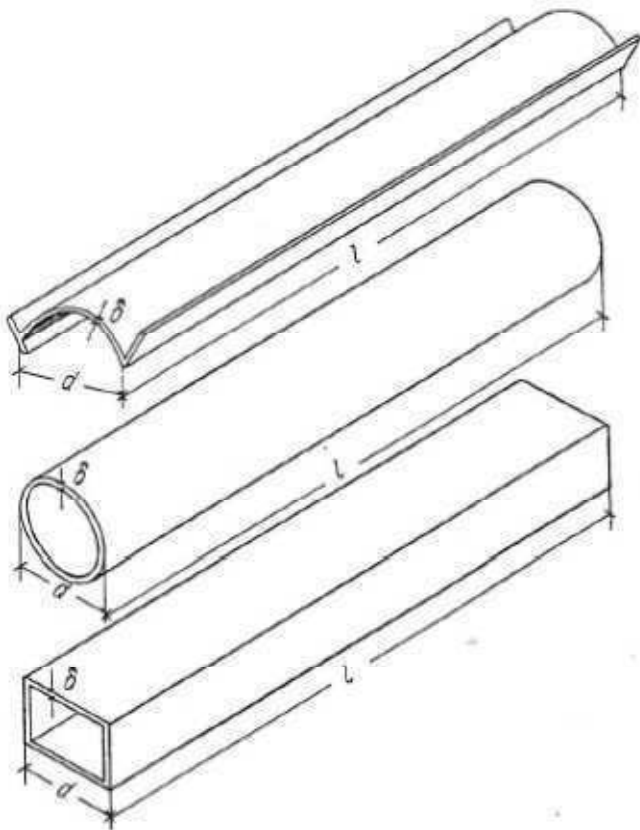
ALU - $\lambda_{\text{lamda}} = 9$ - $\lambda_{\text{lamda barre}} = 2.6$ - flambage en mode 0

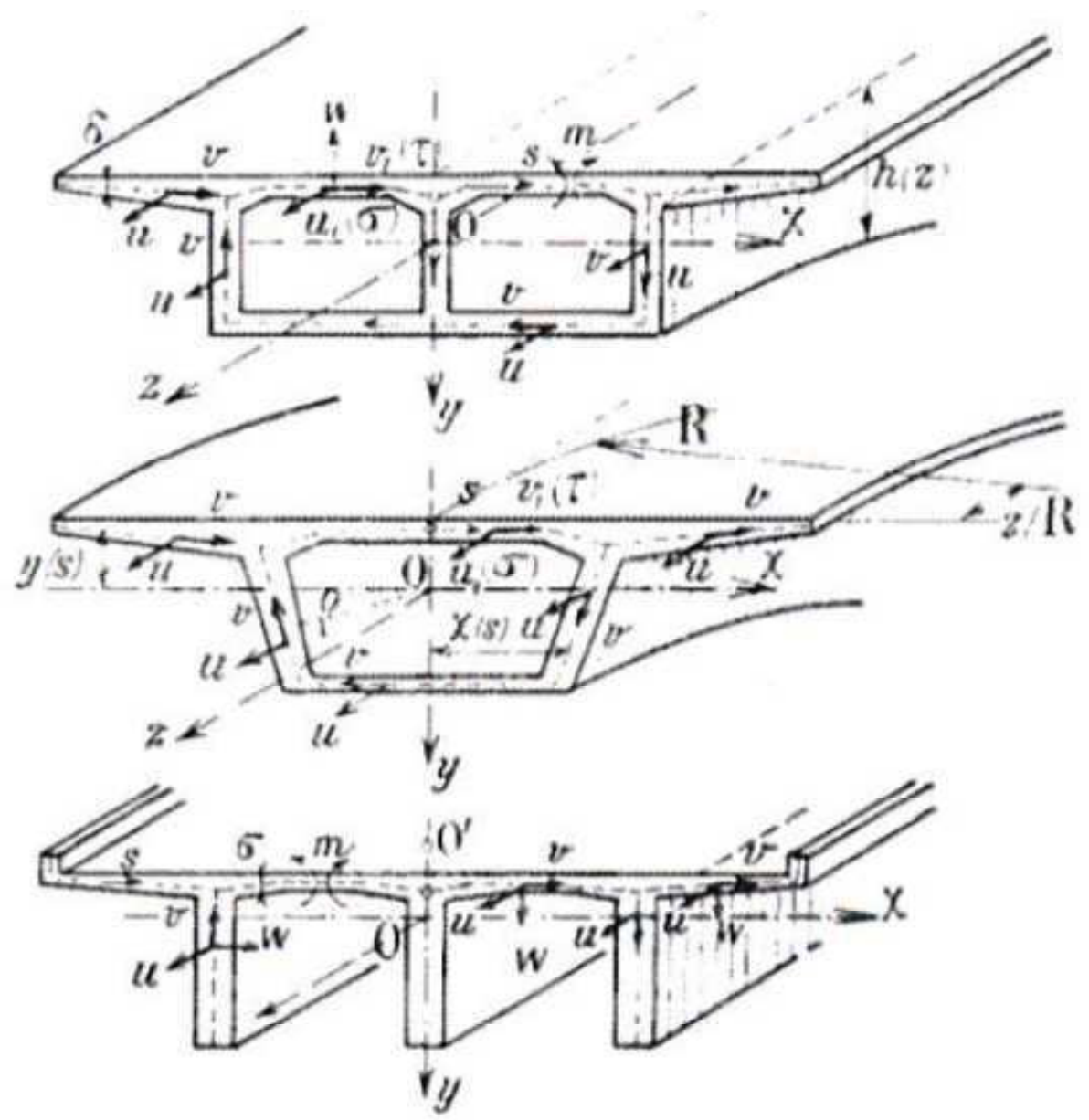
MULTI PARAMETER PROBLEMS

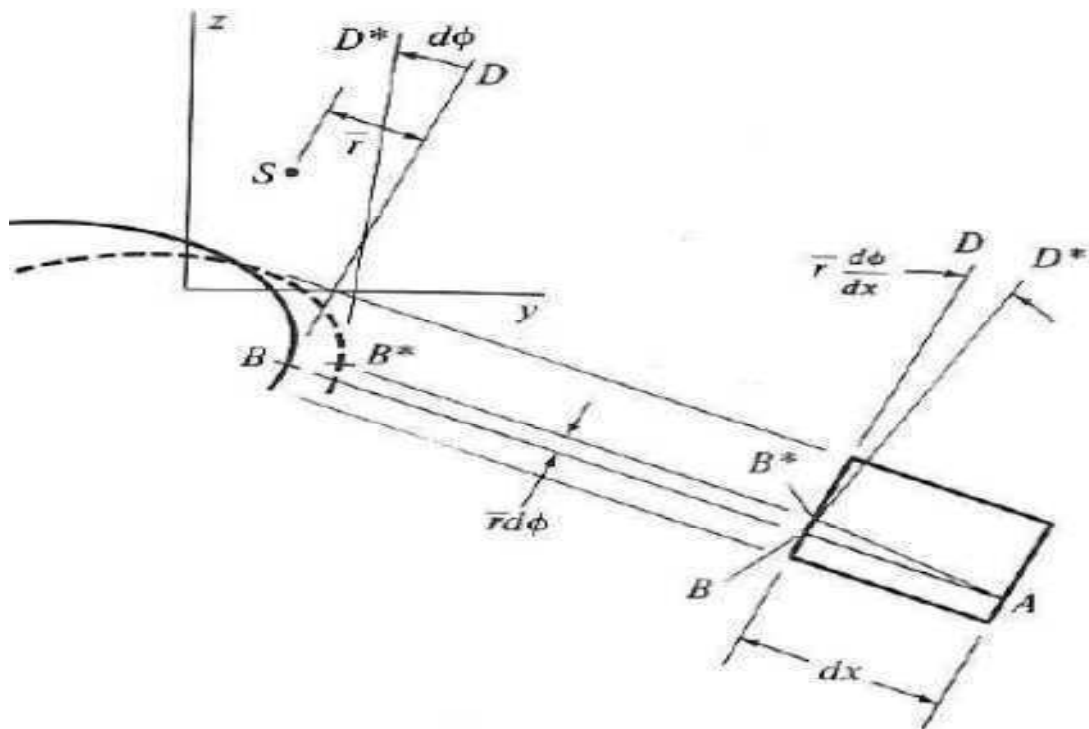
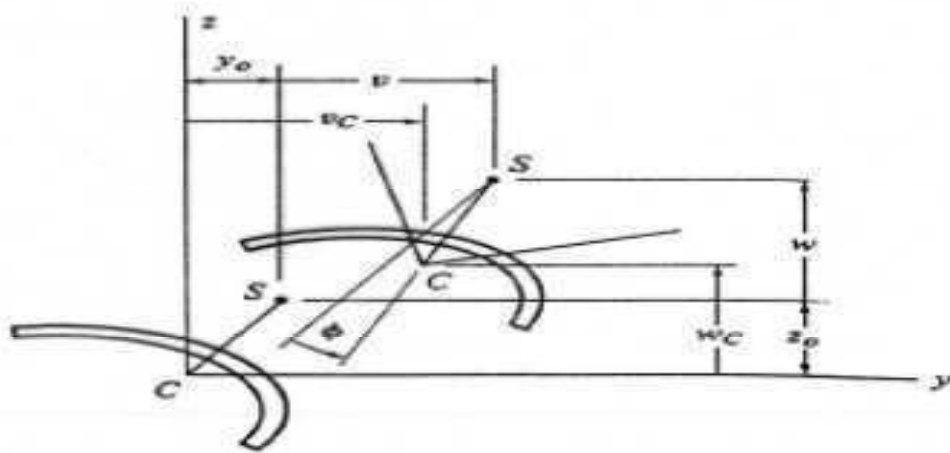
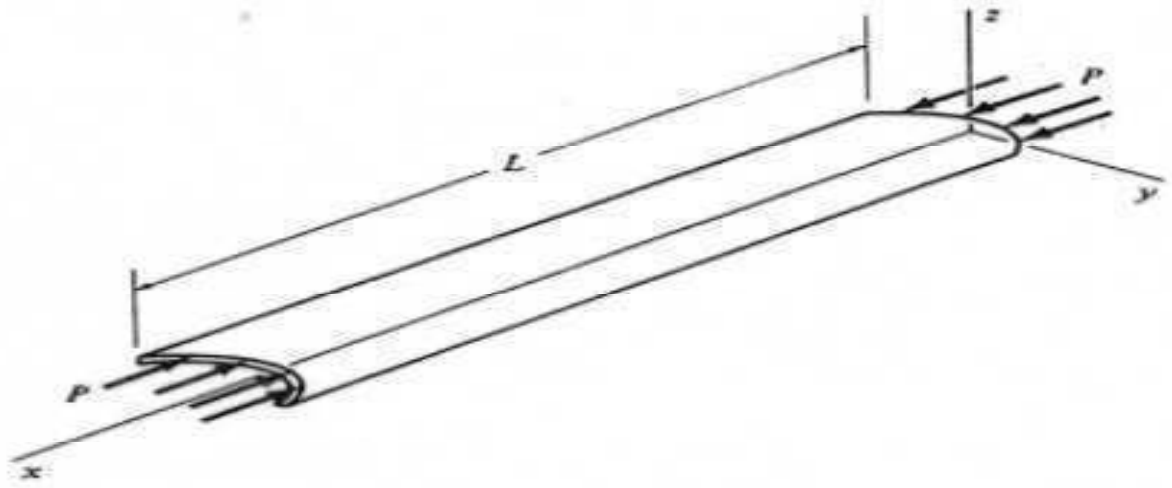


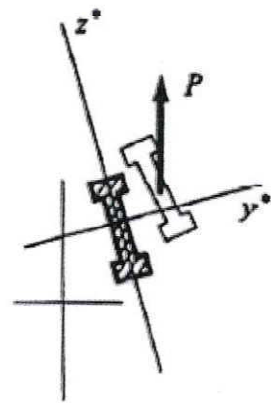
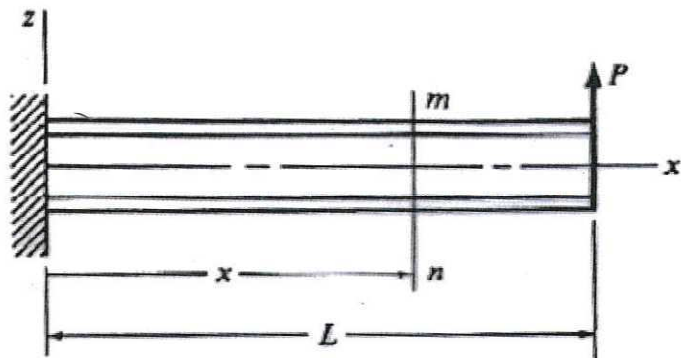
Thin-walled structures

Open or closed cross sections :









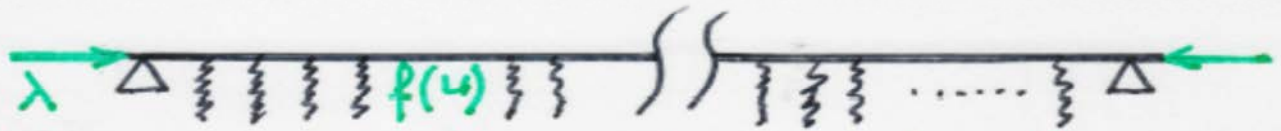
$$E C_w \phi'''' - GJ \phi'' - \frac{P^2}{E I_{33}} (L - x)^2 \phi = 0$$

$$\xi = L - x, \quad k = \frac{P^2}{E I_{33}} GJ$$

$$\rightarrow \frac{d^2 \phi}{d\xi^2} + k^2 \xi^2 \phi = 0$$

$$k = \frac{4.013}{L^2} \quad \text{or} \quad P_{cr} = 4.01 \frac{(E I_{33} GJ)^{1/2}}{L^2}$$

Very long structures



$$f(u) = k_1 u + k_3 u^3 + \dots$$

$$u'''' + \lambda u'' + (\text{sign } k_3) u^3 + u = 0$$

$$u'''' + \lambda u'' + u = 0$$

$$\lambda_c = 2$$

$$\varepsilon = \lambda - 2, \quad x = \varepsilon^{1/2} \kappa$$

$$u = \varepsilon^{1/2} u_1 + \varepsilon u_2 + \dots$$

$$u_1 = A_1(x) e^{ix} + \text{c.c.}$$

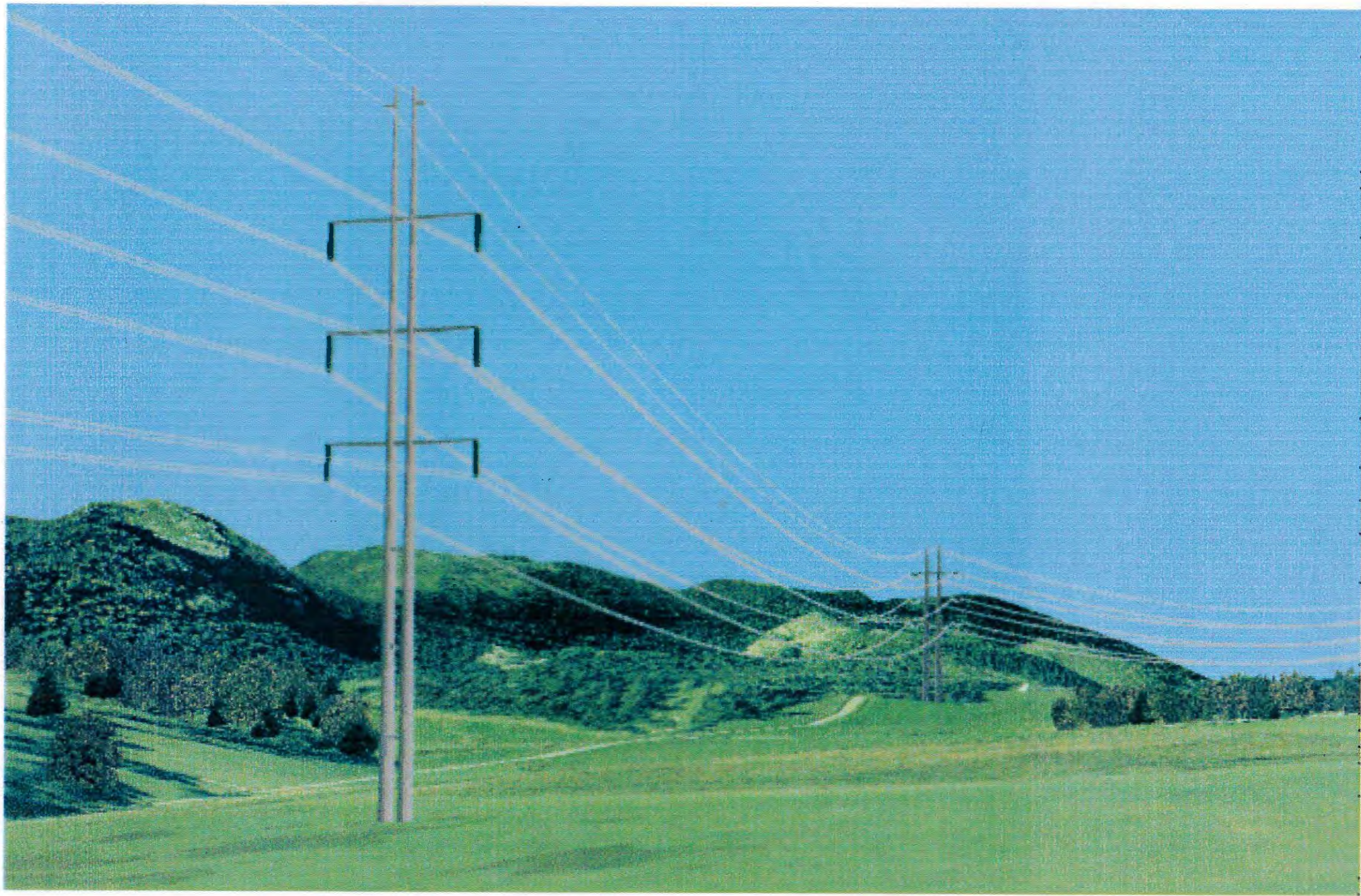
$$A_1(x) = \alpha(x) e^{i\theta(x)}$$

$$4A_1'' + A_1 - A_1 |A_1|^2 = 0$$

$$\alpha(x) = \varepsilon^{1/2} A_1(x)$$

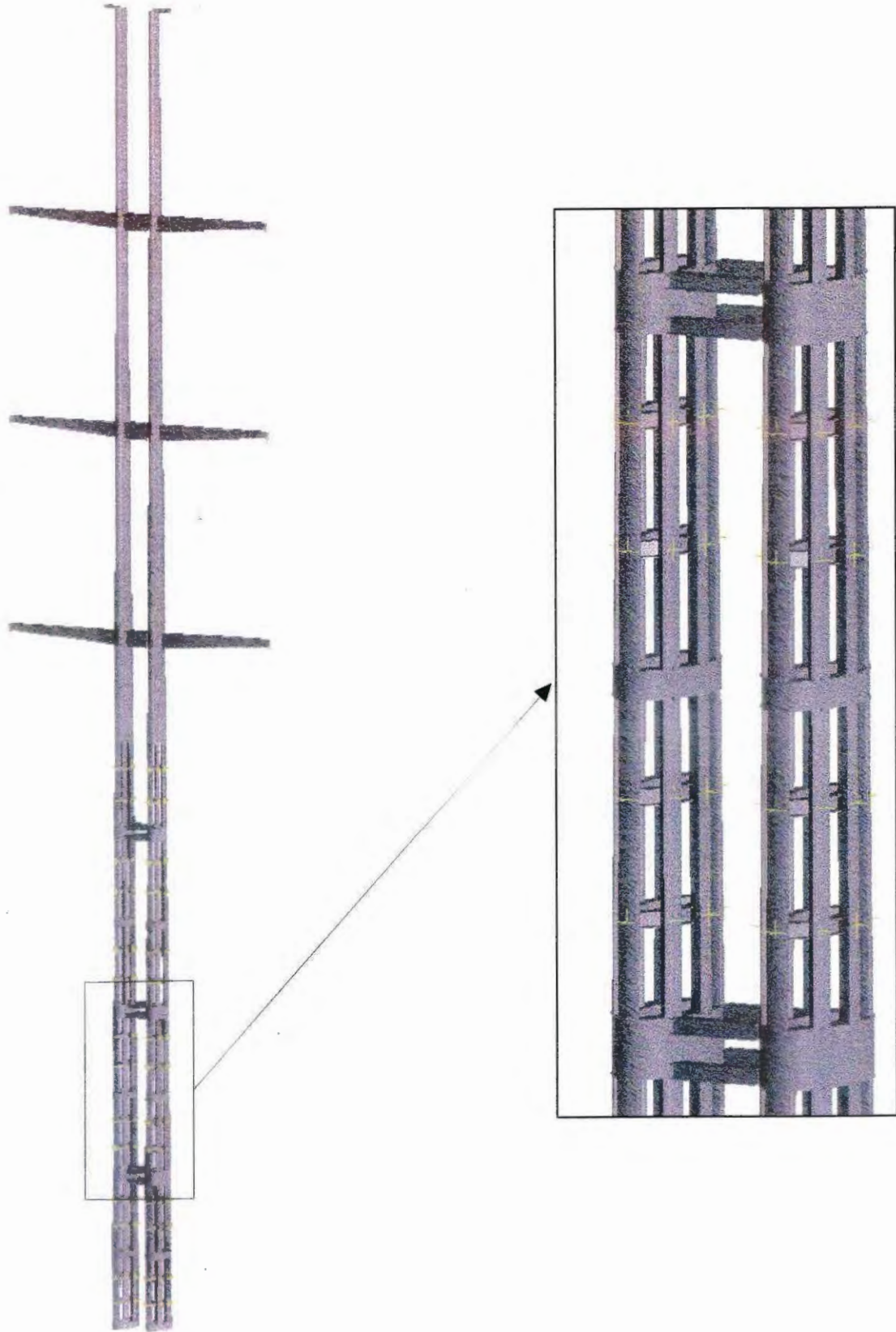
* Amplitude modulation

* Localization

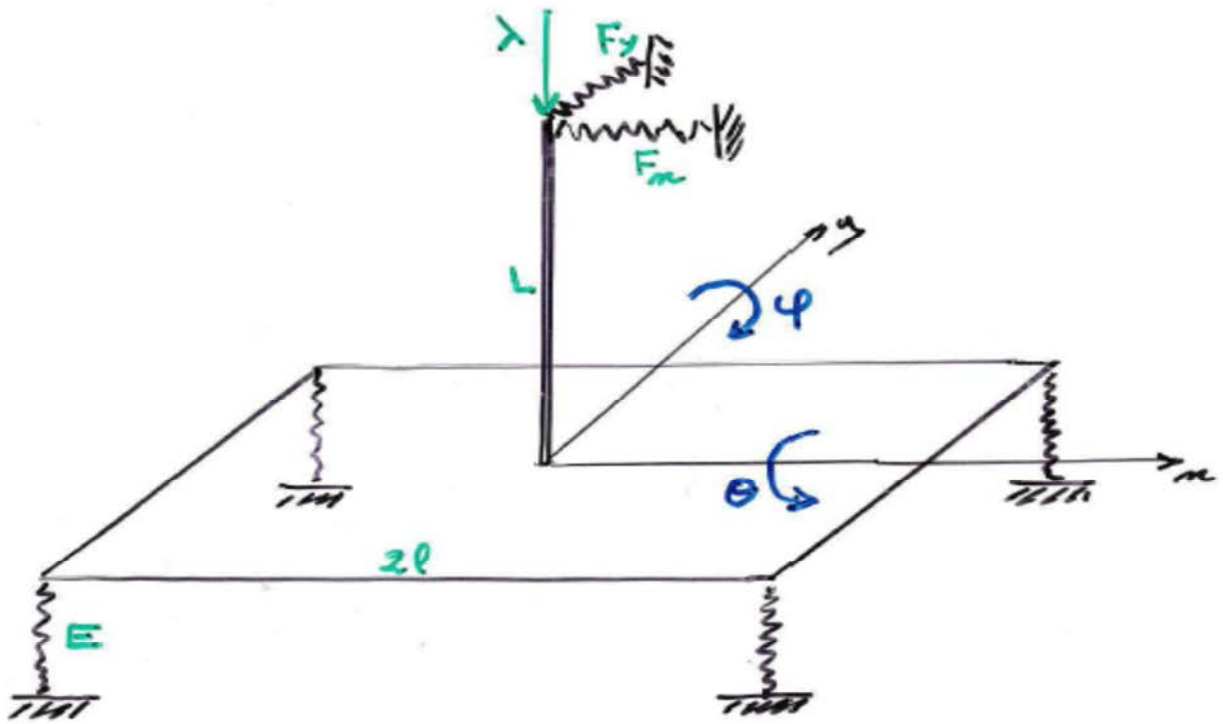




Pylônes MIMRAM

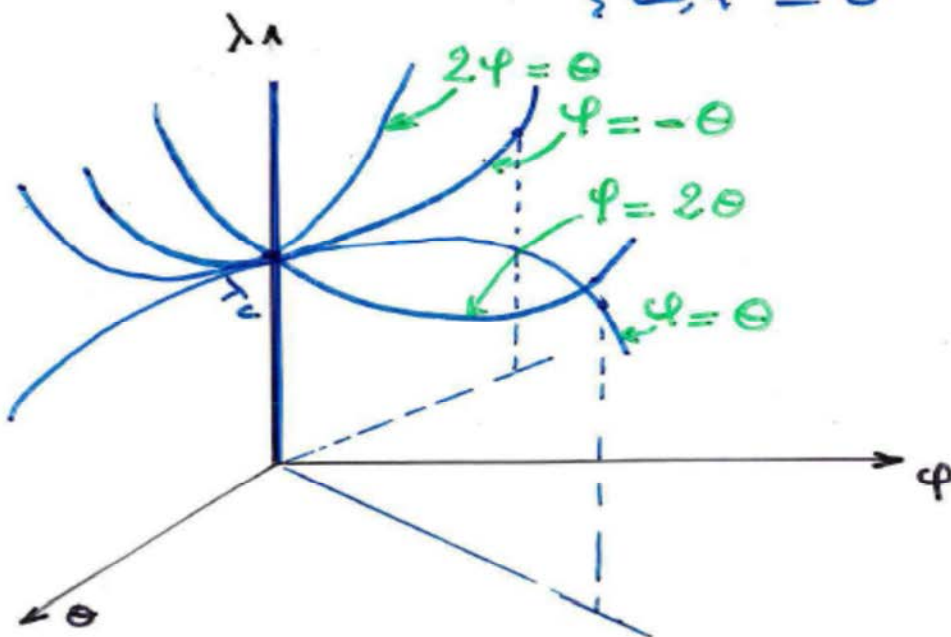


A simple model

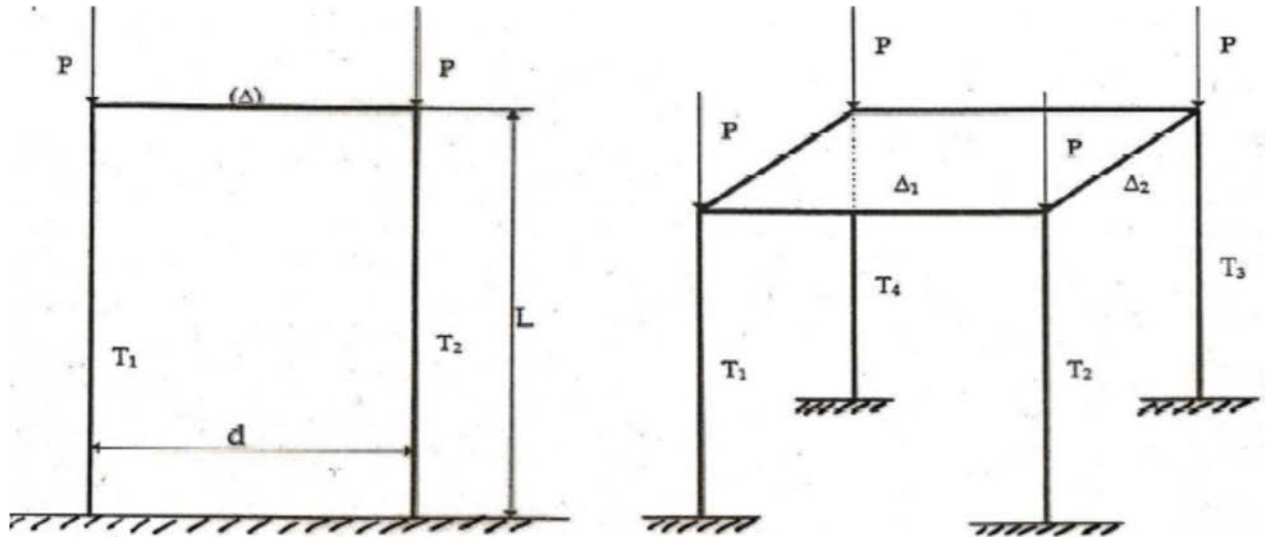


$$E(u, \theta, \varphi, \lambda)$$

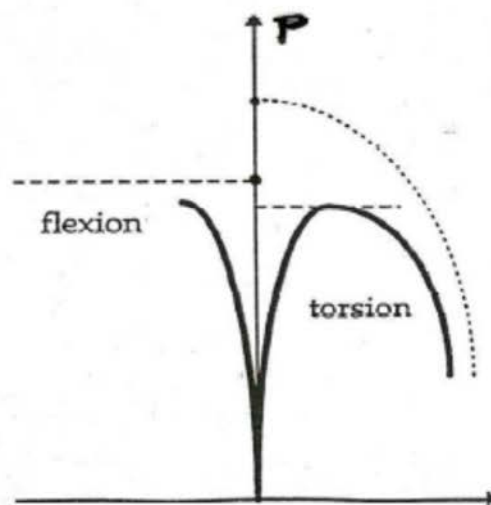
$$\begin{aligned} \left. \begin{array}{l} \text{---} \\ \text{---} \\ \text{---} \end{array} \right\} E, u &= 0 \\ E, \theta &= 0 \\ E, \varphi &= 0 \end{aligned}$$



A slightly more complicated model

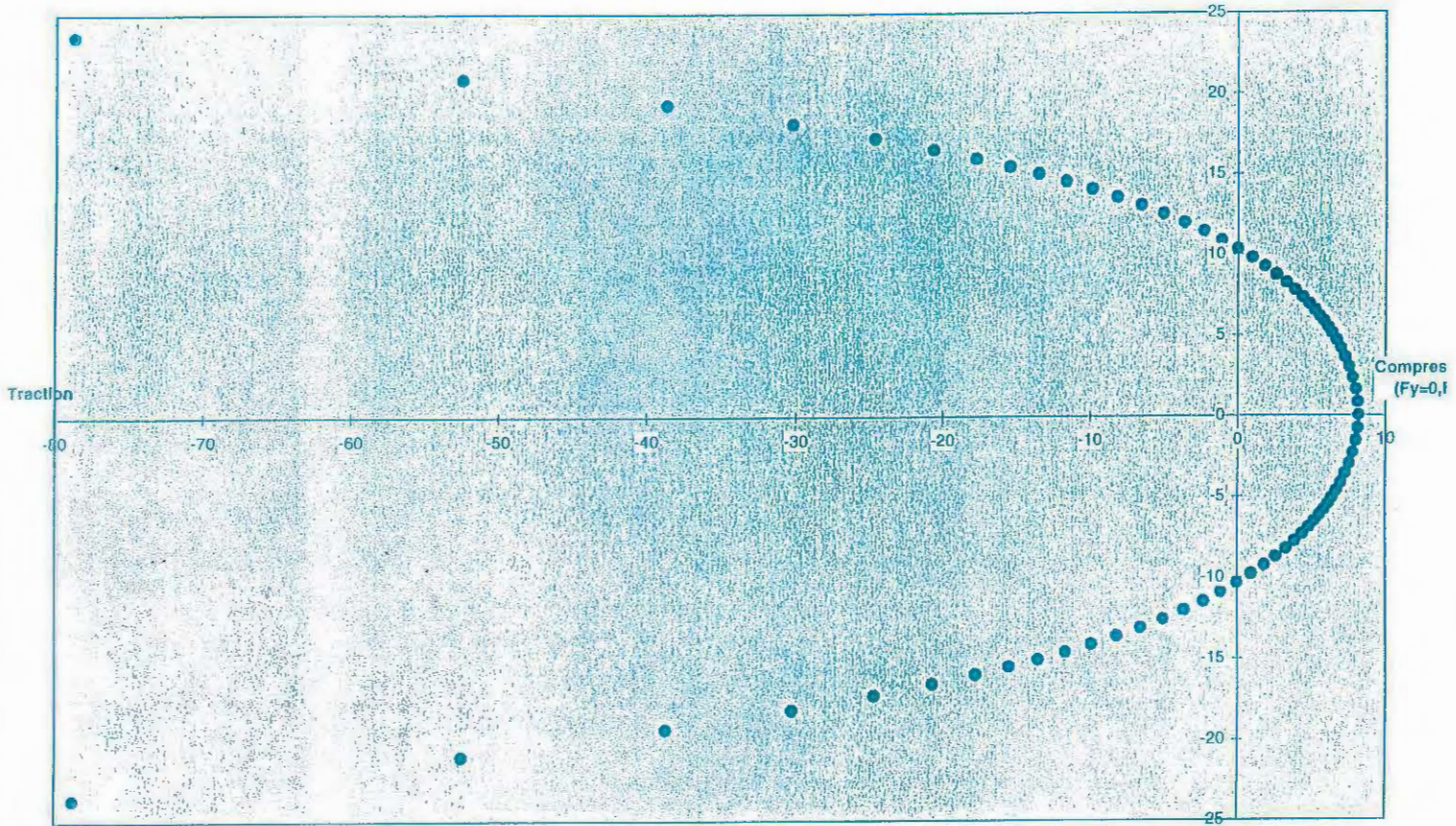


$$\left\{ \begin{array}{l} P_{\sigma}^{Ox} \in \left[\frac{\pi^2}{4L^2} EI_x, \frac{\pi^2}{L^2} EI_x \right] \\ P_{\sigma}^{Oy} \in \left[\frac{\pi^2}{4L} EI_y, \frac{\pi^2}{L} EI_y \right] \\ P_{\sigma}^{tors} \in \left[\min\left(\frac{\pi^2}{L^2} EI_x, \frac{\pi^2}{L^2} EI_y \right), \max\left(\frac{\pi^2}{L^2} EI_x, \frac{\pi^2}{L^2} EI_y \right) \right] \end{array} \right.$$



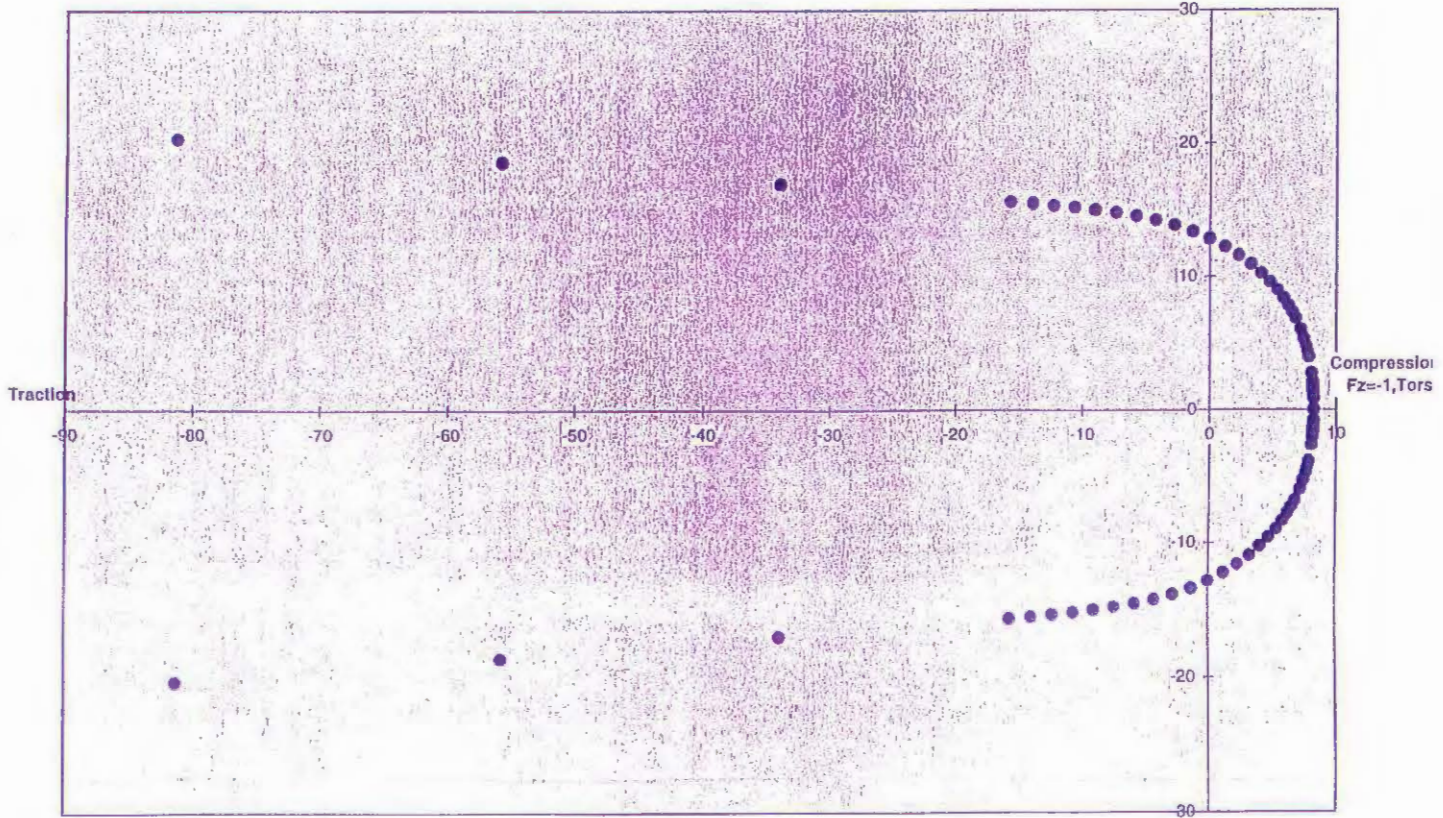
Domaine de flambement Compression/Flexion selon (Ox)

Flexion (N)
($F_y = -1, F_z = 0$)

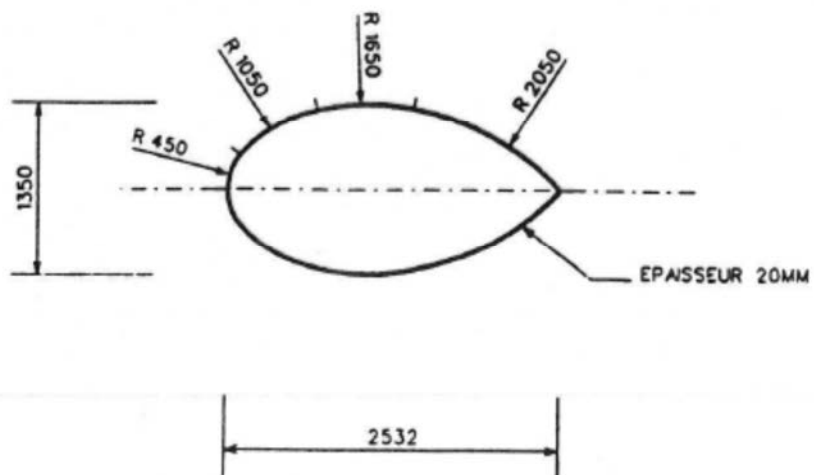
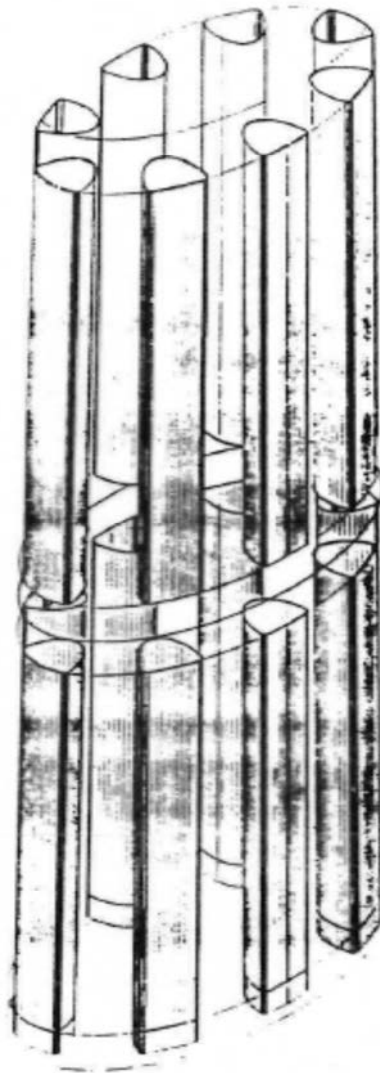


Domaine de stabilité en torsion/compression

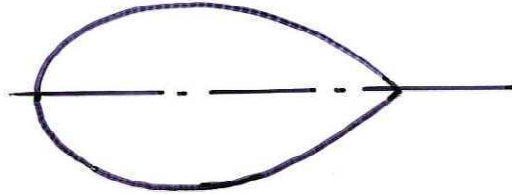
Torsion (N)
 $F_z = 0, Tors = 1$



Choice of the modelling



Poutres Voiles Minces

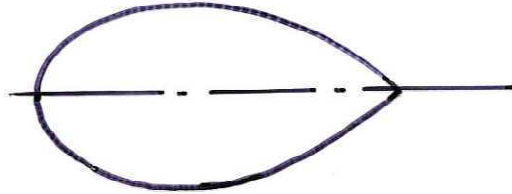


$$\left\{ \begin{array}{l} E I_y w_c^{(4)} + P w_c'' = 0 \\ E I_z v_c^{(4)} + P v_c'' + P z_c \theta_x'' = 0 \\ -M_m' + P \left(z_c^2 + \frac{I_y + I_z}{S} \right) \theta_m'' + P z_c v_c'' = 0 \end{array} \right.$$

$$\begin{aligned} \text{avec } M_m &= G I_c \theta_m' + G (J - I_c) \chi' \\ &= G J \theta_m' - E I_\psi \chi'' \end{aligned}$$

+ C. L. $\left\{ \begin{array}{l} \text{en déplacement de l'axe} \\ \text{en torsion} \\ \text{en gauchissement.} \end{array} \right.$

Poutres Voiles Minces



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... Ou Coque Pliée

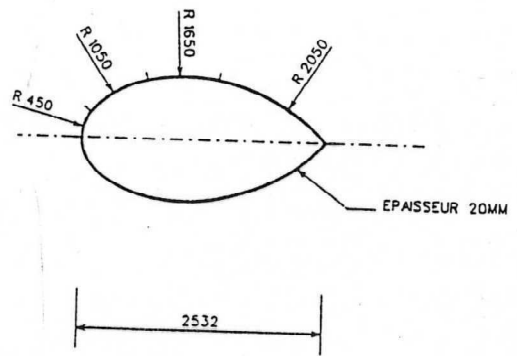
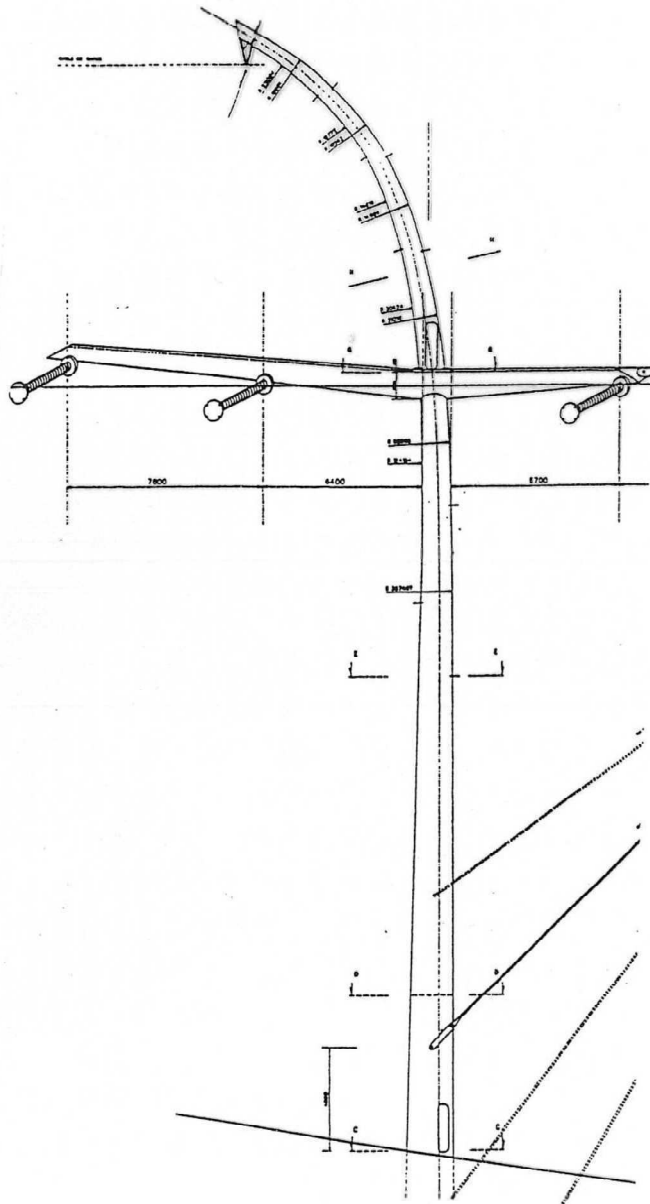
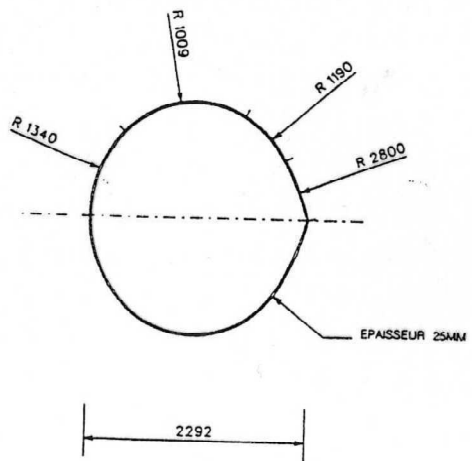
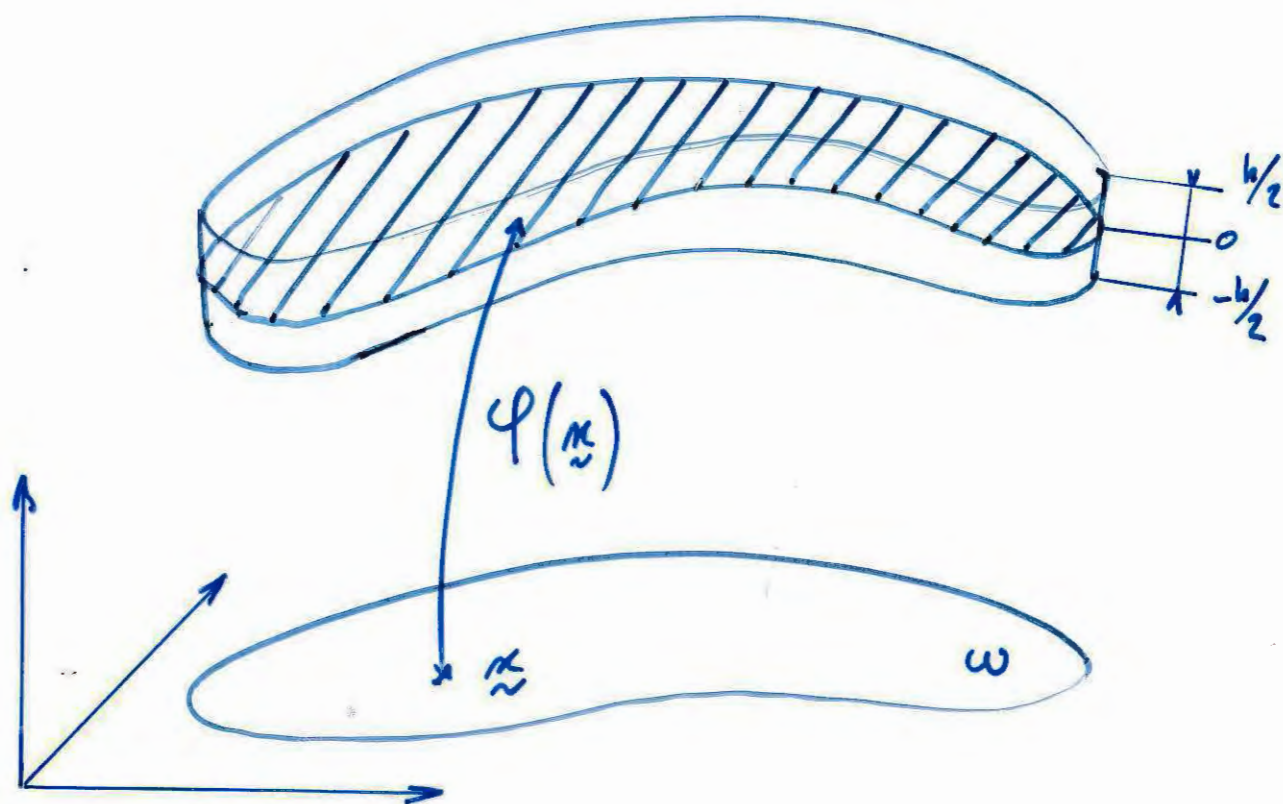


Figure 3-a: Une section du fût en situation courante.





Classically :

$$\varphi \in \mathcal{C}^3(\omega)$$

$$\left\{ \frac{h}{L} \ll 1, \frac{h}{R_i(x)} \ll 1 \right.$$

Then :

$$\mathcal{A}(u, v) = \ell(v)$$



$$\mathcal{A}(u, v) = a^m(u, v) + h^2 a^f(u, v)$$

$$A^{3D}(u, v) = I(v)$$

where:

$$A^{3D}(u, v) = \int_{\omega} \int_{-h/2}^{h/2} G^{\alpha\beta\rho\sigma} \varepsilon_{\alpha\beta}(u) \varepsilon_{\rho\sigma}(v) [1 - 2Hz + Kz^2] \sqrt{a} dx dz$$

exactly!

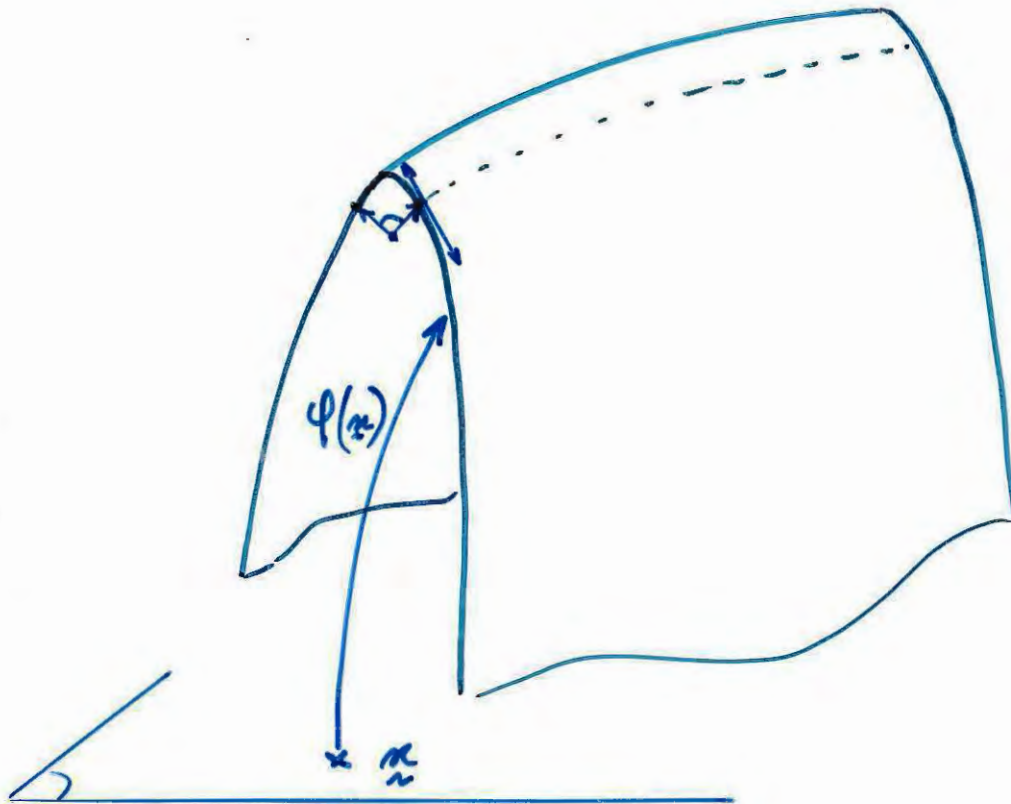
+ Integration over the cross-section

$$\hookrightarrow A^{2D}(u, v) = I(v)$$

$$A^{2D}(u, v) = A^m(u, v) + A^{mf}(u, v) + A^f(u, v)$$

- * Coupling bending / membrane
- * Same limit as "regular" models as $h \rightarrow 0$
- * Limit to a folded shell.

1st Step:



$$\psi \in W^{2, \infty}(\omega)$$

2nd step:

Assumption of very strong curvature

$$\exists \delta > 0, \quad \min_{x \in \bar{\omega}} R_i(x) \geq \frac{h}{2} + \delta$$

(Note the difference with classical thin shells assumption).